

ROOF/FUTURE FLOOR FRAMING PLAN PART A

PLAN NOTES:

1. TOP OF SLAB IS AT ELEVATION = 475.57' UNLESS NOTED.
THIS IS TO BE REFERENCE ELEVATION FOR THIS FLOOR.

2. TOP OF STEEL (BOTTOM OF METAL DECK) IS 3' BELOW TOP OF FLOOR SLAB UNLESS NOTED.

3. STRUCTURAL SLAB TO BE 3' NORMAL WEIGHT CONCRETE OVER 9/16' DEEP x 28 GAGE GALVANIZED FORM DECK (TOTAL THICKNESS = 3') REINFORCED WITH 646 WZ.1 WYZ. 1 WWF.

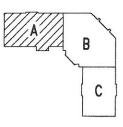
4. TOP OF STEEL ELEVATION OF BEAMS PARALLEL TO JOISTS IS TO MATCH TOP OF JOIST UNLESS NOTED OTHERWISE.

5. STRUCTURAL STEEL FRAMING KEY:

-INDICATES X—BRACING SEMI-RIGID MOMENT CONNECTION SEE 5/S-15 (SEE 1/S-18) TOP OF BEAM RELATIVE-TO NOTE 2







KEY PLAN

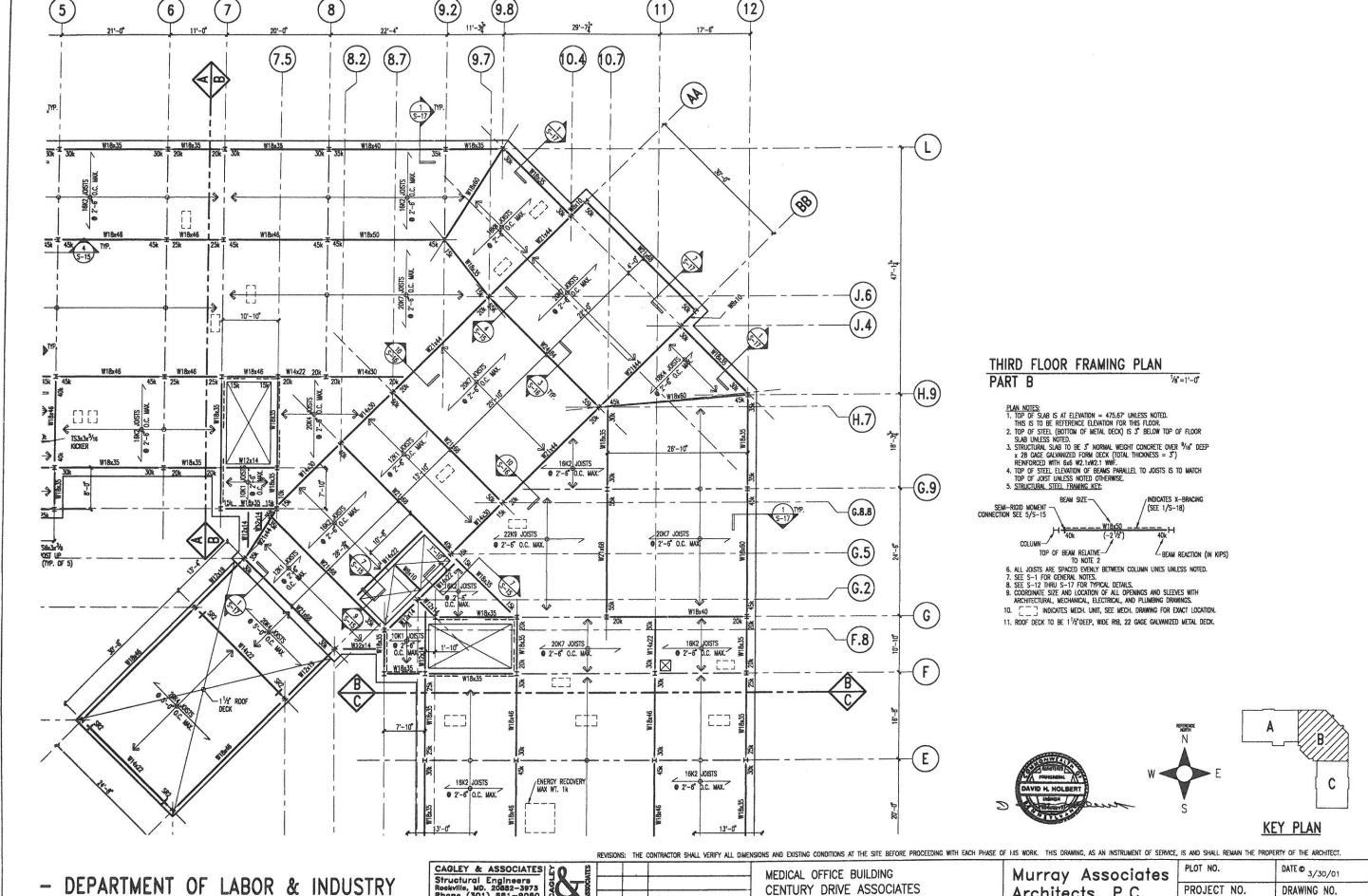
DRAWING NO.

S-8

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REVISIONS: THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AT THE SITE BEFORE PROCEEDING WITH EACH PHASE OF HIS WORK. THIS DRAWING, AS AN INSTRUMENT OF SERVICE, IS AND SHALL REMAIN THE PROPERTY OF THE ARCHITECT. DATE © 3/30/01

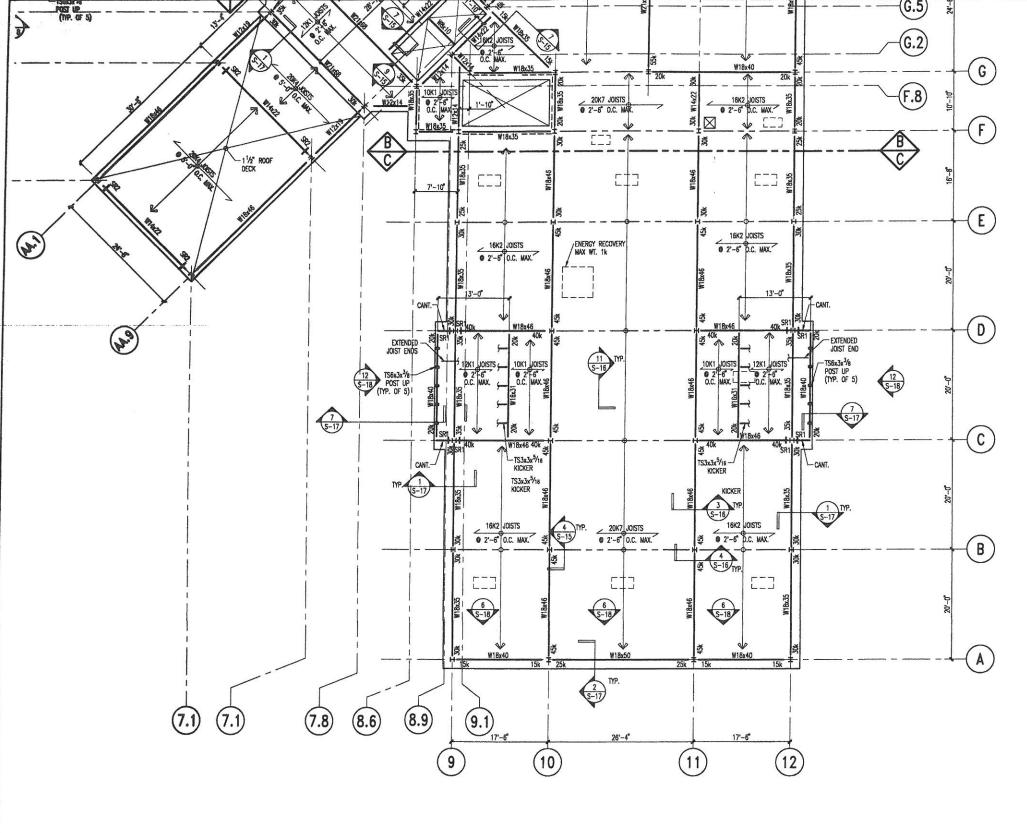
CAGLEY & ASSOCIATES STRUCTURAL Engineers Rockville, ND. 20852–3973 Phone (301) 881–9050	MEDICAL OFFICE BUILDING	Murray Associates	PLOT NO.
Rockville, MD. 20852-3973 Phone (301) 881-9050	CENTURY DRIVE ASSOCIATES	Architects, P.C.	PROJECT NO.
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PROJECT No. 2000 188,00	PART A	717 204 2001 (Idx 204 1201)	



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ROOF/FUTURE FLOOR FRAMING PLAN

PLAN NOTES:

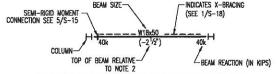
1. TOP OF SLAB IS AT ELEVATION = 475.67" UNLESS NOTED.
THIS IS TO BE REFERENCE ELEVATION FOR THIS FLOOR.

2. TOP OF STEEL (BOTTOM OF METAL DECK) IS 3" BELOW TOP OF FLOOR SLAB UNLESS NOTED.

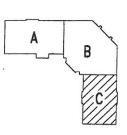
3. STRUCTURAL SLAB TO BE 3" NORMAL WEIGHT CONCRETE OVER 9'16" DEEP × 28 GAGE CALVANIZED FORM DECK (TOTAL THICKNESS = 3") REINFORCED WITH 546 W2.14W12.1 WWF.

4. TOP OF STEEL ELEVATION OF BEAMS PARALLEL TO JOISTS IS TO MATCH TOP OF JOIST UNLESS NOTED OTHERWISE.

5. STRUCTURAL SIEEL FRAMING KEY:







KEY PLAN

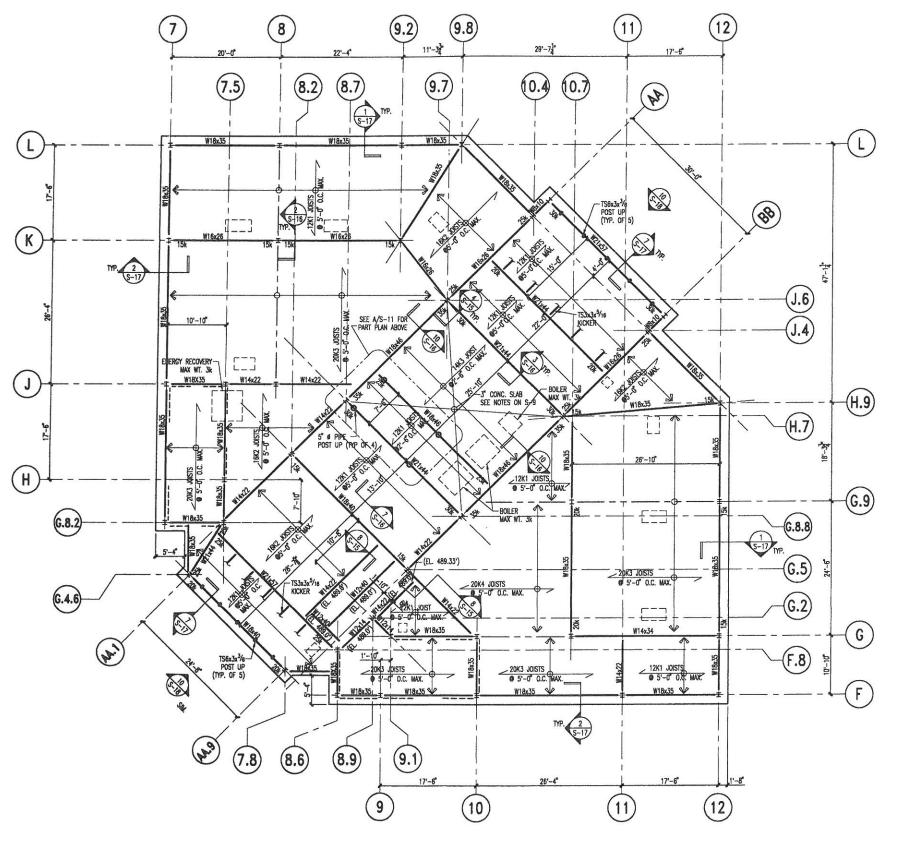
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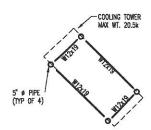
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PROJECT No. 2000 186.00	 TAKTO		





PART PLAN

PLAN NOTES; 1. TOP OF STRUCTURAL STEEL IS 490'-11".

ROOF FRAMING PLAN

PART B

1/8" = 1'-0"

PLAN NOTES:

1. TOP OF STRUCTURAL STEEL (BOTTOM OF METAL DECK) IS 488,67°.

THIS IS TO BE REFERENCE ELEVATION FOR THIS FLOOR.

2. TOP OF STEEL ELEVATION OF BEAMS PARALLEL TO JOISTS IS TO MATCH ELEVATION AND SLOPE OF JOIST BETWEEN SUPPORTING GROERS UNLESS

NOTED.

S. ROOF DECK TO BE 1 ½ DEEP, WIDE RIB, 22 GAGE GALVANIZED METAL DECK.

4. STRUCTURAL STEEL FRAMING KEY:

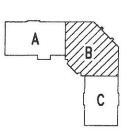
-Indicates X-bracing (SEE 1/S-18) SEMI-RIGID MOMENT-CONNECTION SEE 5/S-15/

5. ALL JOISTS ARE SPACED EVENLY BETWEEN COLUMN LINES UNLESS NOTED.
6. SEE S-1 FOR GENERAL NOTES,
7. SEE S-12 THRU S-17 FOR TYPICAL DETAILS.
8. COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS. INDICATES MECH. UNIT, SEE MECH. DRAWINGS FOR EXACT LOCATION.

10. JOISTS ARE TO BE DESIGNED FOR A LINE LOAD THAT IS PARALLEL AND 3"-0" FROM ALL PERIMETER BEAMS. DL= 90 lb/ft LL= 125lb/ft





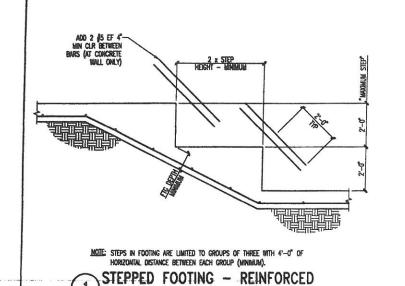


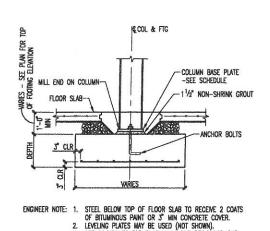
KEY PLAN

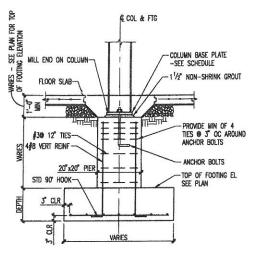
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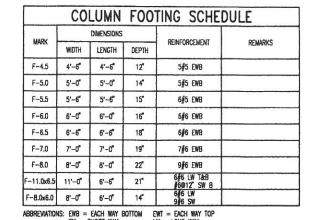
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PROJECT No. 2000 186,00	FAKI D	717 254 2561 (1dx 264 1261)		









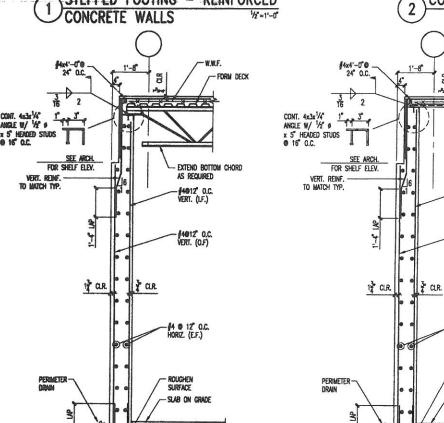
ENGINEER NOTE: 1. STEEL BELOW TOP OF FLOOR SLAB TO RECEIVE 2 COATS OF BITUINHOUS PAINT OR 3" MIN CONCRETE COVER.
2. LEVELING PLATES MAY BE USED (NOT SHOWN).
3. SEE SCHEDULES FOR DIMENSIONS AND REINFORCEMENT.

COLUMN FOOTING WITH PIER (STEEL COLUMN)

COLUMN FOOTING (STEEL COLUMN)

-FORM DECK

PROJECT No. 2000 188.00



HTOIN	SE PLATE S	THICKNESS	A	NCHOR B	DIT		
	LENGTH	TUICKNESS				REMARKS	
		IIIICKIICO	NO.	SIZE	*LENGTH	KEMAKAS	
15"	17	11/4	4	3/4"0	8"		
51/2"	191/2"	11/4"	4	3/4"0	8"		
71/2	191/2"	11/4"	4	3/40	8"		i Lis
51/2"	17"	11/4	4	3/4"0	8"		
17"	17	11/4"	4	3/4"6	8"	SHOWS VARALIBATES	27000
14"	15"	1/2	4	3/40	8"		
	7 1/2 5 1/2 17	7 ½ 19½ 5½ 17 17 17	7 1/2 19 1/2 1 1/4 5 1/2 1 7 1 1/4 1 7 1 7 1 1/4	7 1/2 19 1/2 1 1/4 4 5 1/2 17 1 1/4 4 17 17 1/4 4	7 1/2 19/2 11/4 4 1/46 51/2 17 11/4 4 1/46 17 17/4 4 1/46	7 1/2 19 1/2 1 1/4 4 3/4 8 8 5 1/7 1 1/4 4 3/4 8 8 1 17 1 1/4 4 3/4 8 8 1 17 1 1/4 4 3/4 8 8	7 1/2 19 1/2 1 1/4 4 3/4 8 8 5 1/2 17 1 1/4 4 3/4 8 8 1 17 17 17 17 4 3/4 8 8

* LENGTH INDICATED IS MINIMUM EMBEDMENT.

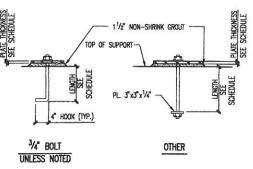
13/4" MIN.L 4 BOLTS

NOTES: 1) SEE SCHEDULE FOR PLATE DIMENSIONS & BOLT SIZES.

2) WELD AS REQUIRED TO DEVELOP FULL MOMENT CAPACITY OF COLUMN.

3) MINIMUM WELD TO BE "FILLET.

W COLUMN BASE PLATE



NOTES: 1) SEE SCHEDULE FOR BOLT SIZE.
2) LEVELING PLATE MAY BE USED AT CONTRACTORS OPTION WITH ENGINEERS APPROVAL.
3) HEAVY WASHERS OR PLATES REQUIRED AT ALL OVERSIZED HOLES.

BASE PLATE SETTING DETAIL

PROJECT NO.

ONE STORY CONCRETE WALL WITH JOIST BEARING

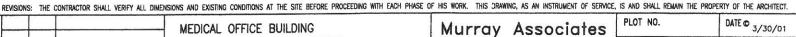
5 ONE STORY CONCRETE WALL
WITH JOIST PARALLEL



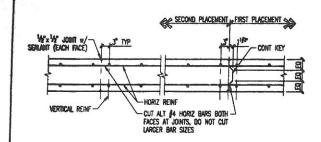
DRAWING NO.

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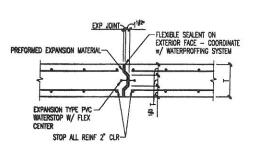
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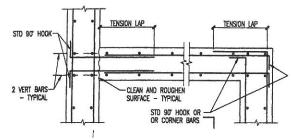
NOTE: 1. PRONDE CONSTRUCTION JOINT OR CONTROL JOINT AT 25'-0" MAX. SPACING.
2. LOCATE FIRST JOINT NO FURTHER THAN 15'-0" FROM CORNER.
3. JOINT LOCATIONS AND DETAILS TO BE APPROVED BY ARCHITECT AND STRUCTURAL ENGINEER.
4. DO NOT USE THIS DETAIL FOR SHEARMALLS OR WALLS DESIGNED TO SPAN HORZONTALLY.
5. PRIOR TO SECOND PLACEMENT OF WALL AT CONSTRUCTION JOINTS, ABRASIVE BLAST OR CHIP FIRST FUNCEMENT FACE OF JOINT TO REMOVE LATTICE HOWEY COMBING, ETC. CLEAN WITH WATER AND STIFF BRUSH AND MAKE SECOND PLACEMENT WHEN CONCRETE IS DAMP.



3/4"=1'-0"

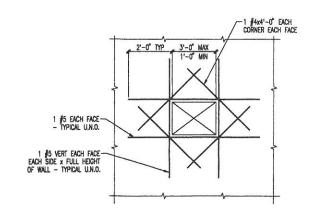


EXPANSION JOINT CONCRETE WALL



DOWEL BARS SAME SIZE AND SPACING AS HORIZONTAL REINFORCING. SEE 5/S-13 FOR TENSION LAP SPLICE SCHEDULE. CORNER BARS MAY BE SUBSTITUTED FOR 97 HOOKS ON ENDS OF HORIZONTAL BARS FOR EACH LAYER OF REINFORCING. LENGTH OF EACH LEG TO BE TENSION LAP PER DETAIL 5/S-13.

3 REINFORCING AT WALL CORNERS



OPENING IN CONCRETE WALL

COADE	60 BARS			CONC	RETE S	TRENGT	H, PSI				
GRADE	OU BARS	30	000	40	000	50	000	60	000		
BAR SI	CENTER TO CENTER BAR SPACING ZE	LESS	4db OR MORE (NOTE 3)	LESS THAN 4db	4db OR MORE (NOTE 3)	LESS THAN 4db	4db OR MORE (NOTE 3)	LESS THAN 4db	4db OR MORE (NOTE 3)	2db	4d
#3	TOP BARS	1'-9"	1'-9"	1'-6"	1'-6"	1'-4"	1'-4"	1'-4"	1'-4"	3/,"	11/
#3	OTHER BARS	1'-4"	1'-4"	1"-4"	1'-4"	1'-4"	1'-4"	1'-4"	1'-4"	1/4	17
#4	TOP BARS	2'-6	2'-4"	2'-2"	2'-0"	1'-11"	1'-10"	1'-9"	1'-9"	48	n ^a
#*	OTHER BARS	1'-11"	1'-10"	1'-8"	1'-7	1'-6"	1'-5"	1'-5"	1'-4"	1	2
#5	TOP BARS	3'-10"	2'-11"	3'-4"	2'-6"	3'-0"	2'-3"	2'-9"	2'-1"	11/4"	1/
#2	OTHER BARS	3'-0"	2'-5	2'-7	1'-11"	2'-4"	1'-9"	2'-2"	1'-8"		21/
#6	TOP BARS	5'-5"	3'-6"	4'-g"	3'-0"	4'-3"	2'-9"	3'-10"	2'-6"	.3/*	78
#0	OTHER BARS	4'-2"	2'-8"	3'-8"	2'-4"	3'-3"	2'-1"	3'-0"	1"-11"	13/4"	3
#7	TOP BARS	7'-5"	4'-1"	6'-5"	3'-6"	5'-9"	3'-2"	5'-3"	2"-11"	.3/#	-11
# '	OTHER BARS	5'-9"	3'-2	4'-11"	2'-9"	4'-5"	2'-5"	4'-1"	2'-3	13/4"	31/
#8	TOP BARS	9'-9"	4'-11"	8'-6"	4'-3"	7'-7	3'-10"	6'-11"	3'-6"	ngi	, 12
#0	OTHER BARS	7'-6"	3'-9"	6'-6"	3'-3"	5'-10"	2'-11"	5'-4"	2*-8*	2"	4
#9	TOP BARS	12'-4"	6'-2"	10'-9"	5'-4"	9'-7	4'-10"	8'-9"	4'-5"	01/2	.1/
#3	OTHER BARS	9'-6	4'-9"	8'-3"	4'-2"	7'-5"	3'-8"	6'-10"	3'-5"	21/4"	41/
#10	TOP BARS	15'-8"	7'-10"	13'-7"	6'-10"	12'-2"	6'-1"	11'-1"	5'-7"	1/2	۳
710	OTHER BARS	12'-1"	6'-1"	10'-6"	5'-3"	9'-4"	4'-8"	8'-6"	4'-3"	21/2	5
#11	TOP BARS	19'-3"	9'-8"	16'-8"	8'-4"	14'-11"	7'-6"	13'-7"	6'-10"	23/4"	-1/
11	OTHER BARS	14'-10"	7'-5"	12'-10"	6'-5"	11'-6"	5'-9"	10'-6°	5'-3"	27/4	51/2

1) THIS DETAIL DOES NOT APPLY TO COLUMN VERTICAL BARS.
2) TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
3) THE USE OF TABULATED VALUES IN THIS COLUMN IS DEPENDENT UPON A MINIMUM CONCRETE COVER AS FOLIOWS: OUTER LAYER BARS IN WALLS AND SLASS REQUIRE 24th MINIMUM COVER, ALL OTHER BARS REQUIRE 4th MINIMUM COVER REQUIREMENTS ARE NOT MET, VALUES IN THE "LESS THAN 446" COLUMN SHALL BE USED.
4) FOR LIGHTWEIGHT AGGREGATE, MULTIPLY ABOVE VALUES BY 1.3.
5) FOR EPOXY-COATED BARS MULTIPLY TOP BARS BY 1.31, OTHER BARS BY 1.50.

TENSION LAP SPLICE LENGTH FOR BEAM, SLAB, WALL AND CONCRETE JOIST REINFORCING BARS

WALL THOMASSO	BAR SIZE & SPA	DEMARKS		
WALL THICKNESS	HORIZONTAL	VERTICAL	REMARKS	
6" TO 8"	#4 @ 12"	∯4 ⊕ 18°	SINGLE LAYER	
81/2" TO 10"	#4 0 18°	#4 9 18"	DOUBLE LAYER	
10 ¹ /2" TO 12"	#4 9 16"	#4 9 18"	DOUBLE LAYER	
12 ¹ /2" TO 14"	#4 @ 14"	# 4 9 18°	DOUBLE LAYER	

NOTE: 1. THE ABOVE SHALL BE MINIMUM REINFORCING AND ARRANGMENTS FOR WALLS UNLESS OTHERWISE DETAILED.
2. ALL SPLICES SHALL BE TENSION LAP SPLICES UNLESS APPROVED OTHERWISE BY THE STRUCTURAL ENGINEER. SEE 5/S-13 FOR SCHEDULE.

6 MINIMUM REINFORCEMENT CONCRETE WALLS



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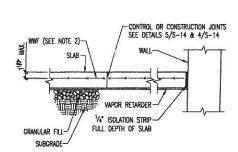
WALL TYPICAL DETAILS

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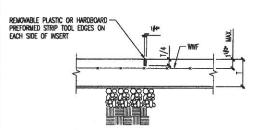
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NOTES: 1) SEE SEQUENCE OF PLACING SLAB ON GRADE FOR LOCATION OF JOINTS.
2) PROMDE SUPPORT CHAIRS TO HOLD WWF AND/OR REINFORCING IN POSITION DURING CONCRETE PLACEMENT.

SLAB ON GRADE

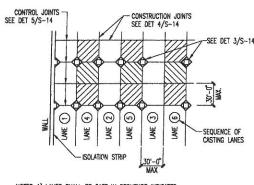


NOTES: 1) FILL JOINT WITH SEALANT AFTER SLAB HAS BEEN CURED.
2) CONSTRUCTION JOINT MAY REPLACE CONTROL JOINT
3) SEE SEQUENCE OF PLACING SLAB ON GRADE FOR

4) SAWCUT JOINTS ARE PERMITTED WITH THE APPROVAL OF THE STRUCTURAL ENGINEER ONLY. IF APPROVED, SAWCUT JOINTS USING A "SOFF-CUT" MACHINE OR EQUAL IMMEDIATELY AFTER FINISHING SLAB,

5) PROVIDE SUPPORT CHAIRS TO HOLD WWF AND/OR REINFORCING IN POSITION DURING CONCRETE PLACEMENT.

SLAB ON GRADE CONTROL JOINT 5 FOR TYPICAL SLABS



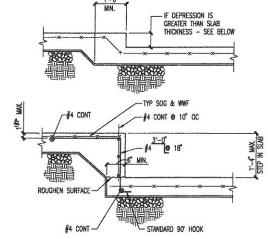
NOTES: 1) LANES SHALL BE CAST IN SEQUENCE INDICATED.

2) LANES SHALL BE DIVIDED BY CONSTRUCTION JOINTS AT COLUMN CENTERLINES AND SUBDIVIDED AT A MAXIMUM OF 30"-0" CENTERS.

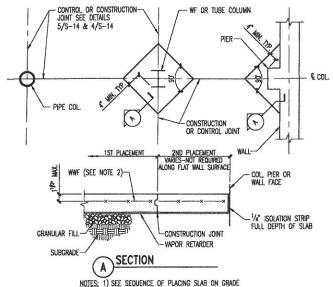
3) IN AREAS WHERE COLUMNS DO NOT OCCUR PROVIDE CONSTRUCTION AND/OR CONTROL JOINTS AS SHOWN.

4) ALLOW 48 HOURS BETWEEN ADJACENT LANE PLACEMENT.

SEQUENCE OF PLACING SLAB ON GRADE

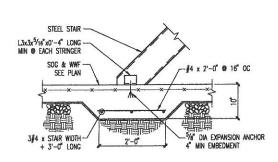


SLAB ON GRADE DEPRESSION AND STEP IN SLAB ON GRADE

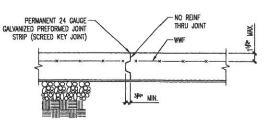


NOTES: 1) SEE SEQUENCE OF PLACING SLAB ON GRADE FOR LOCATION OF JOINTS.
2) PROVIDE SUPPORT CHAIRS TO HOLD WWF AND/OR REINFORCING IN POSITION DURING CONCRETE PLACEMENT.

JOINTS AT COLUMNS AND PIERS FOR SLAB ON GRADE



SLAB ON GRADE AT STEEL STAIR



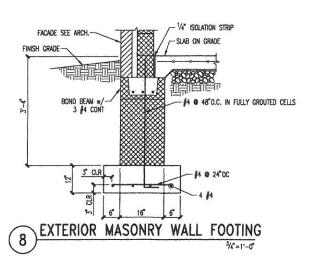
NOTES: 1) CONSTRUCTION JOINT MAY REPLACE CONTROL JOINT 2) REFER TO ARCHITECTURAL DETAILS FOR JOINT FILLER WHERE REQUIRED.

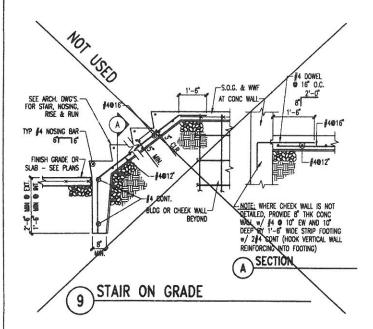
3) SEE SEQUENCE OF PLACING SLAB ON GRADE FOR LOCATION OF JOINTS.

4) PROVIDE SUPPORT CHAIRS TO HOLD WHY AND/OR REJECTORY DESCRIPTION OF JOINTS.

REINFORCING IN POSITION DURING CONCRETE PLACEMENT

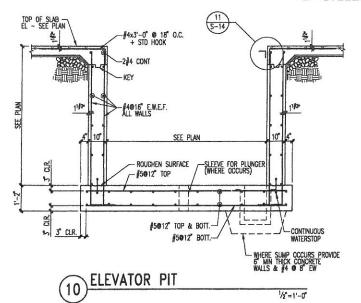
SLAB ON GRADE CONSTRUCTION JOINT

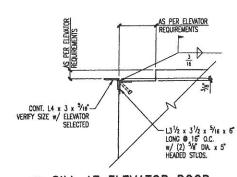




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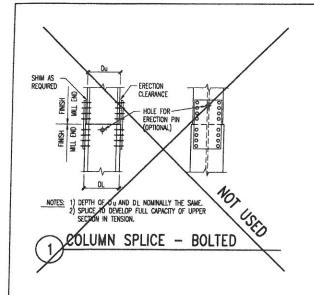




SILL AT ELEVATOR DOOR CONCRETE WALL OR BEAM

REVISIONS: THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AT THE SITE BEFORE PROCEEDING WITH EACH PHASE OF HIS WORK. THIS DRAWING, AS AN INSTRUMENT OF SERVICE, IS AND SHALL REMAIN THE PROPERTY OF THE ARCHITECT.

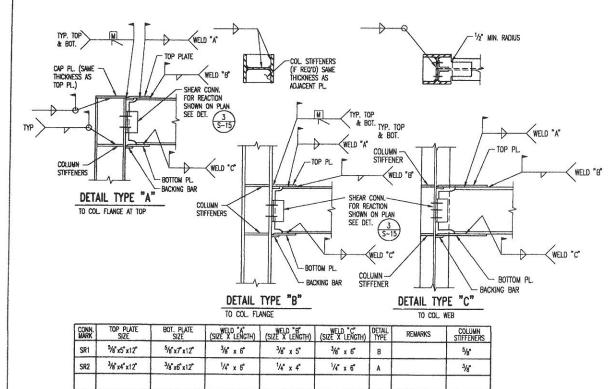
DATE © 3/30/01 CAGLEY & ASSOCIATES & 0 MEDICAL OFFICE BUILDING Murray Associates Structural Engineers
Rockville, MD. 20852-3973
Phone (301) 881-9050 CENTURY DRIVE ASSOCIATES PROJECT NO. DRAWING NO. Architects, P.C. 1600 North Second Street Harrisburg, PA. 17102-2499 717-234-2581 (fax 234-1201) COPYRIGHT 1998, CAGLEY & ASSOCIATES S - 14SLAB ON GRADE DETAILS PROJECT No. 2000 188.00



NOTES : DETAILING OF STRUCTURAL STEEL CONNECTIONS MUST BE CONSISTENT WITH RECOGNIZED, PUBLISHED METHODS SUCH AS IN THE AISC "ENGINEERING JOURNAL".

- STRUCTURAL STEEL CONNECTIONS HAVE BEEN DESIGNED
 AND DETAILED BY THE ENGINEER OF RECORD AS INDICATED ON
 THESE PLANS AND ACCOMPANYING GENERAL NOTES.
- 2) ALTERNATIVE CONNECTION DETAILS MAY BE SUBMITTED ON SHOP DRAWINGS BY THE CONTRACTOR ONLY IF ACCOMPANIED BY COMPLETE STRUCTURAL CACCULATIONS PREPARED AND SCALED BY AN ENGINEER, LICENSED IN THE PROJECT'S JURISDICTION. FAILURE TO SUBMIT SUCH CALCS. FOR REVIEW CONCURRENT WITH SHOP DRAWING ERECTION PLANS AND DETAILS WILL BE CAUSE FOR REJECTION OF THAT SUBMITTAL.
- 3) CALCS. FOR DETAILS MUST SHOW A RATIONAL ANALYSIS OF A COMPLETE LOAD PATH, INCLUDING LOCAL EFFECTS ON WEBS, FLANCES, ETC. OF THE CONNECTED MEMBERS AND THE DEVICES (PLATES, SEATS, BRACKETS, BOLTS, WEBS, ETC.) AFFECTING ALL CONNECTIONS.

CONNECTION DETAIL NOTES STRUCTURAL STEEL



5 BEAM TO COLUMN CONNECTION
TYPE 3 - SEMI-RIGID

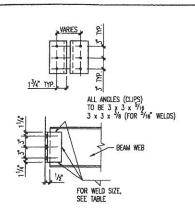
TOTAL NUMBER OF BOLTS	MINIMUM MAXIMUM CONNECTION CONNECTION ON BEAM ON BEAM		3/4" DIA. BOLT CAPACITY (KIPS)		E70xx WELD CAPACITY (KIPS) NOTE 1					
IN CLIPS		A325-N	A325-X NOTE 2	3/16	MIN. WEB	1/4	MIN. WEB	5/16	MIN. WEB	
2	W5,W6	W5,W6	18.6	22.7	18.9	.17	27.8	.26	34.8	,32
4	W8,W10,W12	W8,W10	37.2	45.4	25.4	.26	34.0	.35	42.5	.44
6	W14,W16,W18	W12,W14	55.8	68	40.7	.28	53.5	.37	66.2	.46
8	W21,W24	W16	74.4	91	55.5	.29	74.2	.39	90.8	.48
10	W27,W30	W18	93.0	113	70.0	.30	94.9	.41	116	.50
12	W33,W36	W21	112	136	84.8	.31	114	.42	141	.52

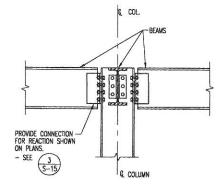
MULTIPLY LISTED CAPACITY BY RATIO OF ACTUAL THICKNESS TO LISTED
MINIMUM THICKNESS.

2) MINIMUM WEB THICKNESS (A36) TO DEVELOP BEARING = .204"

NOTE: DO NOT USE THIS DETAIL FOR COLUMNS LESS THAN 8" x 8"

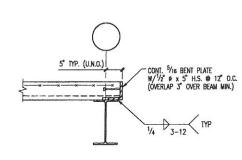
TYPICAL FRAMED BEAM CONNECTIONS



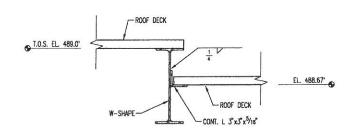


NOTE: SEE PLAN & SCHEDULES FOR SIZES & LOCATION.

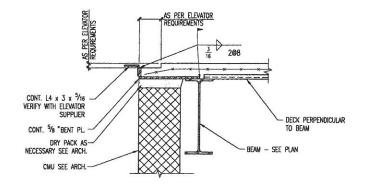
BEAM TO COLUMN CONNECTION



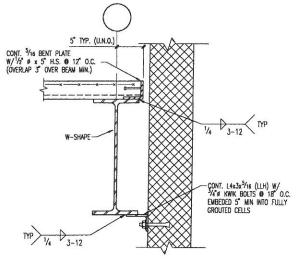
TYPICAL EDGE OF SLAB AT OPENING



DETAIL





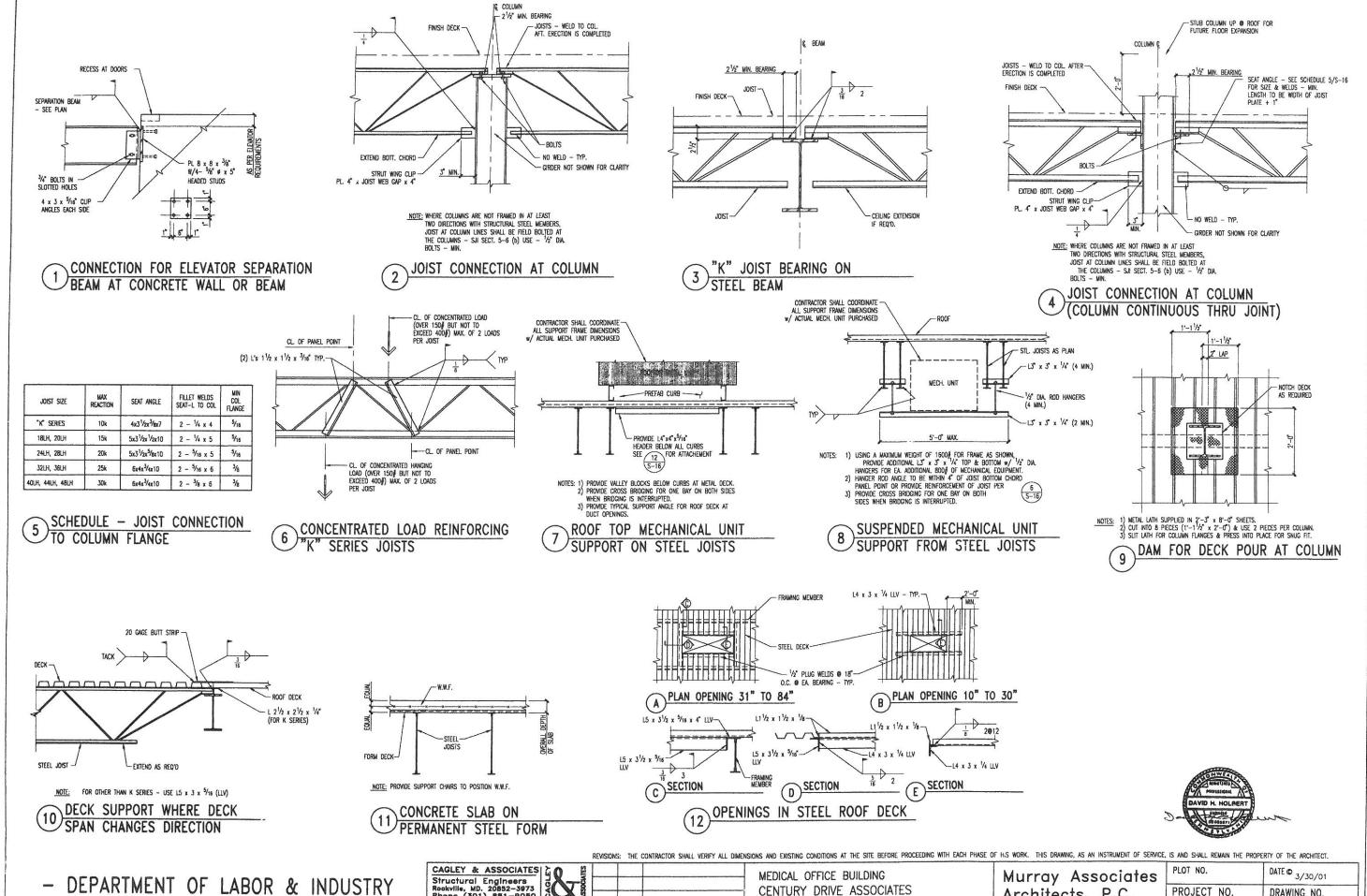


EDGE OF SLAB AT 9 SHAFT WALL



- DEPARTMENT OF LABOR & INDUSTRY
- APPROVAL

	KENDIONO:	THE CONTRACTOR SHALL VERIFT ALL DIMEN	ISIUNS AND EXISTING CONDITIONS AT THE SITE BEFORE PROCECUING WITH EACH FINSE O	THIS WORK. THIS DIGNING, AS AN INSTRUMENT OF SERVICE	, IS AND SIMLE REMAIN THE PROP	ENTI OF THE ANCHITECT.
CAGLEY & ASSOCIATES &			MEDICAL OFFICE BUILDING	Murray Associates	PLOT NO.	DATE © 3/30/01
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PROJECT No. 2000 186.00					L	



- APPROVAL

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